

# UCAN UTB TZ ASTER® TORPEDO® BOLT

TECHNICAL MANUAL
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## **▶ DESCRIPTION**

UCAN UTB TZ Patented Aster Thread TORPEDO BOLT is an excellent anchoring solution for medium-duty applications. TORPEDO® Bolt screw anchors are comprised of a body with a hex flange head. The anchor is manufactured from carbon steel and is heat-treated. It features a 0.0021-inch (53 μm) Mechanical galvanized zinc coating, as per ASTM B695, Class 65, Type 1. TORPEDO® Bolt screw anchors are used as anchorage to resist static, wind, and seismic (Seismic Design Categories A through F) tension and shear loads in cracked and uncracked average weight and lightweight concrete having a specified compressive strength, f'c, of 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

The anchor is tested by an accredited testing agency (American Association for Laboratory Accreditation) in compliance with:

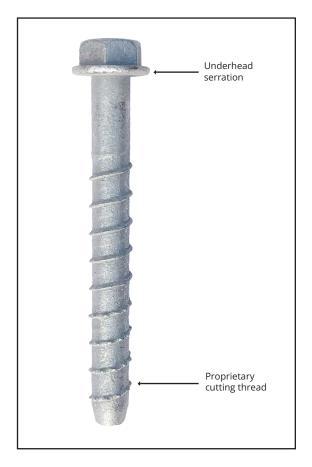
- ASTM E488 ("Standard Test Methods for Strength of Anchors in Concrete and Masonry Elements")
- ICC-ES AC193 ("Acceptance Criteria for Mechanical Anchors in Concrete Elements
- ACI 355.2 ("Qualification of Post-Installed Mechanical Anchors in Concrete")

UTB TZ TORPEDO® BOLT has been evaluated by an accredited independent agency (ICC-ES) for recognition of use in cracked and uncracked concrete, including seismic and wind loads in concrete members having a specified compressive strength, f'c, from 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa). The UTB TZ TORPEDO bolt is listed in the ICC-ES Report ESR4596.

Matched with a standard UCAN ANSI tolerance drill bit, this fastener exhibits consistently high load values. The UCAN TORPEDO® BOLT installs quickly using the UCAN TST Installation Driver, leaving the working surface with the clean appearance of a finished hex washer head.

#### **► FEATURES**

- Use with UCAN standard ANSI compliant drill bit
- Fast installation and reduced edge distance requirements, compared to mechanical expansion anchors.
- One piece fastener with finished hex flange head and locking under-head serration
- Unique thread pattern facilitates ease of installation
- Anchor can be set with an impact or manual socket wrench.
- Removable-Ideal for temporary anchoring applications.
- Anchor size is stamped on head for easy identification and enhanced quality control after anchor Installation.



#### ► LISTING AND APPROVALS



ICC-ES<sup>®</sup>
ESR 4596

UTB 38134, UTB 38212, UTB 383, UTB 384, UTB 385, UTB 12212, UTB 123, UTB 124, UTB 125, UTB 126

#### ► TYPICAL APPLICATIONS

- Racking, Railing, Sill plates, Stadium seating.
- Anchoring equipment

#### **► LIMITATIONS**

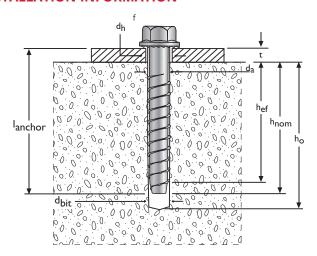
Not recommended for installation into uncured concrete (less than 7 days old).



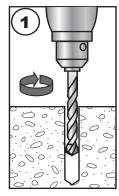
#### ► MATERIAL SPECIFICATIONS

| Properties           | Carbon Steel   |  |  |
|----------------------|--|--|--|
| Anchor body          | Heat treated carbon steel                                      |  |  |
| Head style           | Hex flange head with locking serrations                        |  |  |
| Corrosion protection | Mechanically galvanized as per<br>ASTM B-695, Class 55, Type 1 |  |  |

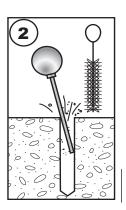
#### **► INSTALLATION INFORMATION**



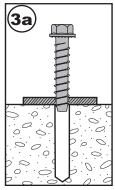
#### ► INSTALLATION INSTRUCTION¹



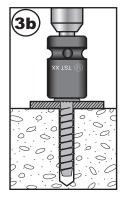
Drill hole to the specified diameter and depth. It is advised to over drill the depth by at least ½". Do not ream the holes.



Blow out dust from the hole



Place the anchor through the fixture



Attach the recommended size TST Torpedo® setter to a 1/2" drive impact wrench, place the setter over the head of the anchor and install. Immediately stop when the setter spins/disengages from the anchor's head.

<sup>&</sup>lt;sup>1</sup> When using impact wrench, there is a risk of over-tightening and damaging the screw, impact tool may not correlate properly with the above setting torques. Over torquing can damage the base material, anchor and/or reduce its holding capacity. If the TST Setter is not used, immediately stop when the bottom of the anchor head comes in contact with the fixture. Use a calibrated hand torque wrench to finish the installation.



# ► TECHNICAL DATA FOR CARBON STEEL UTB FOR LIMIT STATE / STRENGTH DESIGN IN CRACKED AND UNCRACKED CONCRETE

TABLE 1- TORPEDO BOLT SCREW ANCHOR INSTALATION INFORMATION<sup>1</sup>

| Characteristic                      | Symbol                  | Units  | Nominal And                   | hor Diameter                      |
|-------------------------------------|-------------------------|--------|-------------------------------|-----------------------------------|
| Characteristic                      | Symbol                  | Offics | ³/ <sub>8</sub> -inch         | <sup>1</sup> / <sub>2</sub> -inch |
| Nominal Anchor Diameter             | da                      | in     | 3/8                           | 1/2                               |
| Nominal Anchor Diameter             | U <sub>a</sub>          | (mm)   | (9.5)                         | (12.7)                            |
| Nominal Drill Bit Diameter          | d <sub>bit</sub>        | in     | <sup>3</sup> / <sub>8</sub>   | 1/2                               |
|                                     | Ubit                    | (mm)   | (9.5)                         | (12.7)                            |
| Nominal Embedment Depth             | h <sub>nom</sub>        | in     | 3                             | 3                                 |
|                                     | IInom                   | (mm)   | (76)                          | (76)                              |
| Effective Embedment Depth           | h <sub>ef</sub>         | in     | 2.30                          | 2.28                              |
| Effective Effibedifient Depth       | Hef                     | (mm)   | (58)                          | (58)                              |
| Minimum Hala Danth                  | hhole                   | in     | 3 <sup>1</sup> / <sub>4</sub> | 3 <sup>1</sup> / <sub>4</sub>     |
| Minimum Hole Depth                  | Thole                   | (mm)   | (83)                          | (83)                              |
| Fixture Hole Diameter               | $d_f$                   | in     | 1/2                           | 5/8                               |
| Fixture Hole Diameter               | $u_f$                   | (mm)   | (12.7)                        | (15.9)                            |
| Maximum Installation Torque         | T <sub>inst,max</sub>   | ft.lb  | 25                            | 55                                |
| Waxiii uii iiistallation Torque     | I inst,max              | (Nm)   | (34)                          | (75)                              |
| Maximum impact wrench torque rating | T <sub>impact.max</sub> | ft lb  | 380                           | 380                               |
| Maximum impact wrench torque rating | I impact.max            | (Nm)   | (515)                         | (515)                             |
| Minimum Concrete Thickness          | h <sub>min</sub>        | in     | 4¾                            | 4 1/2                             |
| Willimiditi Concrete Thickness      | IImin                   | (mm)   | (121)                         | (114)                             |
| Critical Edge Distance              |                         | in     | 5                             | 4                                 |
| Critical Edge Distance              | Cac                     | (mm)   | (127)                         | (102)                             |
| Minimum Edge Distance               | <u> </u>                | in     | 2                             | 2                                 |
| willing tage distance               | C <sub>min</sub>        | (mm)   | (51)                          | (51)                              |
| Minimum Spacing                     | <b>.</b>                | in     | 3                             | 3                                 |
| Minimum Spacing                     | Smin                    | (mm)   | (76)                          | (76)                              |

<sup>&</sup>lt;sup>1</sup>The tabulated data is to be used in conjunction with the design criteria given in ACI 318-(19 and -14) Chapter 17 or ACI 318-11 Appendix D or for Canadian design CSA A23.3-19 Annex D as applicable.

TABLE 2 - RESISTANCE FACTORS FOR LIMIT STATE DESIGN IN ACCORDANCE WITH CSA A23.3-14, ANNEX D

| Setting information  | Symbol     | Units   | Nominal Anchor Diameter 3/8" and 1/2" |
|--|------------|---------|---------------------------------------|
| Concrete material resistance factor                        | Фс         | -       | 0.65                                  |
| Steel material resistance factor                           | $\phi_{S}$ | -       | 0.85                                  |
| Strength reduction factor for tension, steel failure modes | R          |         | 0.80                                  |
| Strength reduction factor for shear, steel failure modes   | R          |         | 0.75                                  |
| Strength reduction factor for tension, concrete            | R          | Cond. A | 1.15                                  |
| failure modes  | , A        | Cond. B | 1.00                                  |
| Strength reduction factor for Shear, concrete failure      | R          | Cond. A | 1.15                                  |
| modes  | I N        | Cond. B | 1.00                                  |



## TABLE 3 - STRENGTH DESIGN DATA IN CRACKED AND UNCRACKED NORMAL WEIGHT CONCRETE<sup>1,2,3,4</sup>

|  | Symbol                   | Units                   | Nominal Anchor Diameter |                    |
|--|--------------------------|-------------------------|-------------------------|--------------------|
| Characteristic   |                          |                         | ³/ <sub>8</sub> -inch   | ¹/₂-inch           |
| Nominal Embedment Depth  | h <sub>nom</sub>         | in                      | 3                       | 3                  |
| Anchor Category  | 1, 2 or 3                | (mm)                    | (76)<br>1               | (76)<br>1          |
| Anchor category  | 1,2013                   |                         | 1                       | ı                  |
| Steel St   | trength in Ter           | sion and She            |                         |                    |
| Minimum specified ultimate strength  | $f_{uta}$                | psi<br>(N/mm²)          | 147,000<br>(1,014)      | 147,000<br>(1,014) |
|  |                          | psi psi                 | 117,600                 | 117,600            |
| Minimum specified yield strength   | $f_{y}$                  | (N/mm²)                 | (811)                   | (811)              |
| Effective stress area (screw anchor body)  | Ase                      | in <sup>2</sup>         | 0.103                   | 0.193              |
| Effective stress area (serew afferior body)  | 7 ise                    | (mm²)                   | (66.5)                  | (124.5)            |
| Steel Strength in Tension  | N <sub>sa</sub>          | lb<br>(kN)              | 12875<br>(57.3)         | 24,125<br>(107.3)  |
| Strength Reduction Factor for Steel Failure in Tension   | φ                        | (KIN)                   | 0.65                    | 0.65               |
| Stiength Reduction Factor for Steel Failure III Tension  | Ψ                        | - Ile                   | FF17                    |                    |
| Steel Strength in Shear  | $V_{sa}$                 | lb<br>(kN)              | 5517<br>(24.54)         | 6.570<br>(29.2)    |
|  |                          | lb                      | 5517                    | 6.570              |
| Steel Strength in Shear, Seismic   | $V_{sa,eq}$              | (kN)                    | (24.54)                 | (29.2)             |
| Strength Reduction Factor for Steel Failure in Shear   | φ                        | -                       | 0.60                    | 0.60               |
| Bul  | lout Strength            | in Tonsion <sup>3</sup> |                         |                    |
|  |                          | lb                      |                         |                    |
| Pullout Strength in Uncracked Concrete   | $N_{p,uncr}$             | (kN)                    | =                       | -                  |
| Pullout Strength in Cracked Concrete   | $N_{p,cr}$               | lb                      | -                       | -                  |
| - and at on ongar mended controlled  | 110,0                    | (kN)                    |                         |                    |
| Pullout Strength in Cracked Concrete, Seismic  | $N_{p,eq}$               | lb<br>(kN)              | -                       | -                  |
|  | _                        |                         |                         |                    |
| Concrete   | Breakout Sti             | ength in Ten            | 2.30                    | 2.28               |
| Effective embedment  | $h_{ef}$                 | (mm)                    | 2.30<br>(58 mm)         | 2.28<br>(58)       |
| Effectiveness Factor for Uncracked Concrete  | <b>k</b> <sub>uncr</sub> | -                       | 27                      | 27                 |
| Effectiveness Factor for Cracked Concrete  | k <sub>cr</sub>          | -                       | 17                      | 17                 |
| Strength Reduction Factor for Concrete Breakout Strength in Tension  | φ                        | -                       | 0.65                    | 0.65               |
| Axial stiffness in service load range in uncracked concrete  | R                        | lb/inch                 | 246,746                 | 189,880            |
| wigh stilliess in selvice load ralige in unicracked colicrete  | ete β <sub>uncr</sub>    | (N/mm)                  | (43,212)                | (33,250)           |
| Axial stiffness in service load range in cracked concrete  | $oldsymbol{eta}_{cr}$    | lb/inch                 | 177,965                 | 101,150            |
| <del>-</del>   | 1                        | (N/mm)                  | (31,167)                | (17,715)           |
| Concret  | te Breakout S            | trength in Sh           |                         |                    |
| Nominal Diameter   | $d_{o}^{2}$              | in                      | 3/8                     | 1/2                |
|  | 40                       | (mm)                    | (9.5)                   | (12.7)             |
| Load Bearing Length of Anchor  | le                       | in<br>(mm)              | 2.30<br>(58 mm)         | 2.28<br>(58)       |
| Reduction Factor for Concrete Breakout Strength in Shear   | φ                        | - (111111)              | 0.70                    | 0.70               |
| reading in the control of control | . Ψ                      |                         | 5.70                    | 0.70               |
|  | ete Pryout Str           | ength in She            |                         |                    |
| Coefficient for Pryout Strength  | k <sub>cp</sub>          | -                       | 1.0                     | 1.0                |
| Reduction Factor for Pryout Strength in Shear  | $\phi$                   | -                       | 0.70                    | 0.70               |

¹The tabulated data is to be used in conjunction with the design criteria given in ACI 318(-19 and -14) Chapter 17 or ACI 318-11 Appendix D, as applicable.

All values of  $\phi$  were determined from the load combinations of 2021 IBC Section 1605.1, 2018, 2015 and 2012 IBC Section 1605.2, ACI 318 (-19 and -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable. If the load combinations of ACI 318-11Appendix C are used, then the appropriate value of φ must be determined in accordance with ACI 318-11 D.4.4. For reinforcement that meets ACI 318 (-19 and -14) Chapter 17 or ACI 318 Appendix D, as applicable, requirements for Condition A, see ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, for the appropriate φ factor when the load combinations of 2021 IBC Section 1605.1 or 2018, 2015 and 2012 IBC Section 1605.2, ACI 318(-19 and -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used.

<sup>&</sup>lt;sup>3</sup>Where no value is reported for pullout strength, this resistance does not need to be considered.
<sup>4</sup>For limit State Design as per CSA A23.3-19 Annex D, material resistance factors (Φ) and resistance modification factor (R) listed in Table 3 shall be used.